ICC-ES Evaluation Report

ESR-4520

Issued   December 2023

Subject to renewal   December 2024

DIVISION: 05 00 00 — METALS	REPORT HOLDER: OEG BUILDING MATERIALS, INC.	EVALUATION SUBJECT: B-DECK, N-DECK, 1.5", 2", 3" COMPOSITE DECK AND 9/16" AND 3" FORM-DECK	
Section: 05 31 00 — Steel Decking			
Section: 05 31 13 — Steel Floor Decking			
Section: 05 31 23 — Steel Roof Decking			

1.0 EVALUATION SCOPE

Compliance with the following codes:

■ 2021 *International Building Code®* (IBC)

Property evaluated:

■ Structural

2.0 USES

B-Deck, N-Deck, 1.5", 2", 3" Composite Deck and 9/16" and 3" Form-Deck are used as roof and floor decks to support code-required vertical loads.

3.0 DESCRIPTION

3.1   Steel Decks:

3.1.1   B-Deck and N-Deck: B-Deck and N-Deck are cold-rolled from steel sheets complying with ASTM A653 Grade 40 or 50 with either a G60 or G90 galvanized coating. The deck are available in four design thicknesses: 0.0295 in, 0.0358 in, 0.0474 in and 0.0598 in. For deck profiles, see Figure A.

3.1.2   Composite Deck: 1.5", 2", 3" Composite Deck are cold-rolled from steel sheets complying with ASTM A653 Grade 40 with either a G60 or G90 galvanized coating. The 1.5", 2" and 3" Composite Deck are available in four design thicknesses: 0.0295 in, 0.0358 in, 0.0474 in, and 0.0598 in. For deck profiles, see Figure B.

3.1.3   Form-Deck: 9/16" and 3" Form-Deck are cold-rolled from steel sheets complying with ASTM A653 Grade 40 with either a G60 or G90 galvanized coating. The 9/16" Form-Deck is available in 5 design thicknesses: 0.0150 in, 0.0180 in, 0.0240 in, 0.0300 in and 0.0360 in. The 3" Form-Deck is available in four design thicknesses: 0.0295 in, 0.0358 in, 0.0474 in, and 0.0598 in. For deck profiles, see Figure C.

3.2   Concrete Fill:

Concrete must be in accordance with the IBC and must have a minimum 28-day compressive strength of 3,000 psi (20.7 MPa). Concrete must be 145 pcf (2,323 kg/m³) normal weight concrete fill.

4.0 DESIGN AND INSTALLATION

4.1   Design: The design values included in the tables of this report have been determined in accordance with AISI S100 and SDI NC, RD, and C, as applicable, and as required by IBC Section 2210.

4.1.1 Section Properties: The deck and composite deck section properties are provided in Tables 1 and 2 of this report, respectively.

4.1.2 Vertical Load Design: For allowable uniformly distributed loads of B-Deck and N-Deck, see Tables 3A through 3D. For allowable uniformly distributed loads of 1.5", 2" and 3" Composite Deck, see Tables 4A through 4C. For allowable uniformly distributed loads for 9/16" and 3" Form-Deck, see Tables 5A and 5B.

4.1.3 Shear and Web Crippling Strength: The shear and web crippling capacities of deck panels to resist support reactions and concentrated loads are provided in Table 1 of this report.

4.2 Installation:

The B-Deck, N-Deck, 1.5", 2", 3" Composite Deck and 9/16" and 3" Form-Deck must be installed at locations in accordance with the plans and specifications approved by the code official. The steel deck panels must be installed in accordance with this evaluation report and the OEG Building Materials, Inc., published installation guidelines and instructions. If there is a conflict between the OEG Building Materials, Inc. published installation guidelines and instructions and this report, this report governs.

5.0 CONDITIONS OF USE

The B-Deck, N-Deck, 1.5", 2", 3" Composite Deck and 9/16" and 3" Form-Deck described in this report comply with, or are suitable alternatives to what is specified in, the codes listed in Section 1.0 of this report, subject to the following conditions:

5.1 The base metal thickness for steel deck panels delivered to the jobsite must be at least 95 percent of the base (design) metal thickness.

5.2 Special inspection must be provided in accordance with IBC Chapter 17.

5.3 The construction documents prepared or reviewed by a registered design professional, where required by the statutes of the jurisdiction in which the project is to be constructed, specifying the B-Deck, N-Deck, 1.5", 2", 3" Composite Deck and 9/16" and 3" Form-Deck, must indicate compliance with this evaluation report and applicable codes and must be submitted to the code official for approval.

5.4 The minimum loads and deflection limits of IBC Chapter 16 in addition to the construction loads required by the references in IBC 2210.1.1 must be considered by the registered design professional based on the specific occupancy or use, as applicable.

5.5 When the steel deck panels are used as roof decks, the panels must be covered with an approved code-complying roof covering.

5.6 Fasteners used in connections are outside the scope of this report.

5.7 Vertical load design of steel deck panels, without concrete fill, must be based on Table 1 Section properties.

5.8 Concrete-filled sections must not be used to support loads that are predominantly vibratory, such as those for operation of heavy machinery, reciprocating motors or moving loads.

5.9 The B-Deck, N-Deck, 1.5", 2", 3" Composite Deck and 9/16" and 3" Form-Deck are manufactured under a quality control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Steel Deck Roof and Floor Systems (AC43), dated August 2022.

7.0 IDENTIFICATION

7.1 The ICC-ES mark of conformity, electronic labeling, or the evaluation report number (ICC-ES ESR-4520) along with the name, registered trademark, or registered logo of the report holder must be included in the product label.

7.2 In addition, each bundle of steel deck roof and floor panels described in this report is identified by a label bearing the manufacturer's name (OEG Building Materials, Inc.) and the manufacturing location; the deck panel profile name; the minimum base metal thickness (uncoated); the minimum specified yield strength, and the cover width of the panel.

7.3 The report holder's contact information is the following:

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General Notes for Tables:

$I_{xx\text{-}gross}$ —	Gross Moment of Inertia
$I_{xx\text{-}eff+}$ —	Effective Positive Moment of Inertia
$S_{xx\text{-}eff+}$ —	Effective Positive Section Modulus
$I_{xx\text{-}eff\text{-}}$ —	Effective Negative Moment of Inertia
$S_{xx\text{-}eff\text{-}}$ —	Effective Negative Section Modulus
V_{ay} —	Allowable Shear
M_{ax} —	Allowable Bending Moment
I_{d+} -	Hybrid Positive Moment of Inertia
$I_{d\text{-}}$ -	Hybrid Negative Moment of Inertia

TABLE 1—DECK SECTION PROPERTIES (PER FOOT OF WIDTH) [1]

Panel Type	F_y	Design Thickness[2] (in)	Weight (lb/ft)	V_{ay} (lb/ft)	Top in Compression				Bottom in Compression				Hybrid Moment of Inertia per AC43 3.1[3]		Web Crippling Strength[4]	
					$I_{xx\text{-}gross}$ (in⁴)	$I_{xx\text{-}eff+}$ (in⁴)	$S_{xx\text{-}eff+}$ (in³)	M_{ax} (ft-lb)	$I_{xx\text{-}gross}$ (in⁴)	$I_{xx\text{-}eff-}$ (in⁴)	$S_{xx\text{-}eff-}$ (in³)	M_{ax} (ft-lb)	Single Span: I_{d+} (in⁴)	Multiple Span: I_{d-} (in⁴)	End (lb/ft)	Interior (lb/ft)
1.5" B Deck	40	0.0295	1.63	1961	0.180	0.137	0.182	363	0.180	0.173	0.188	376	0.151	0.176	654	1135
1.5" B Deck	40	0.0358	1.97	2358	0.217	0.177	0.226	451	0.217	0.213	0.239	476	0.190	0.214	932	1638
1.5" B Deck	40	0.0474	2.61	3066	0.280	0.253	0.303	605	0.280	0.280	0.313	682	0.262	0.280	1557	2775
1.5" B Deck	40	0.0598	3.28	3793	0.347	0.337	0.384	766	0.347	0.347	0.388	863	0.340	0.347	2381	4283
1.5" B Deck	50	0.0295	1.63	2451	0.180	0.133	0.173	431	0.180	0.167	0.182	454	0.149	0.171	817	1419
1.5" B Deck	50	0.0358	1.97	2947	0.217	0.170	0.223	556	0.217	0.210	0.232	580	0.186	0.212	1165	2047
1.5" B Deck	50	0.0474	2.61	3833	0.280	0.243	0.300	748	0.280	0.280	0.313	781	0.256	0.280	1946	3469
1.5" B Deck	50	0.0598	3.28	4742	0.347	0.327	0.380	948	0.347	0.347	0.388	1057	0.333	0.347	2977	5354
3" N Deck	40	0.0295	2.01	2432	0.833	0.551	0.349	697	0.833	0.757	0.395	788	0.645	0.782	470	964
3" N Deck	40	0.0358	2.43	3582	1.006	0.701	0.457	912	1.006	0.954	0.508	1013	0.803	0.971	677	1377
3" N Deck	40	0.0474	3.22	5702	1.320	1.005	0.650	1298	1.320	1.310	0.711	1419	1.110	1.313	1143	2304
3" N Deck	40	0.0598	4.05	7132	1.645	1.365	0.838	1673	1.645	1.645	0.896	1998	1.458	1.645	1762	3527
3" N Deck	50	0.0295	2.01	2479	0.833	0.536	0.334	834	0.833	0.718	0.365	910	0.635	0.756	588	1205
3" N Deck	50	0.0358	2.43	4005	1.006	0.680	0.436	1088	1.006	0.932	0.491	1225	0.789	0.957	846	1721
3" N Deck	50	0.0474	3.22	7020	1.320	0.967	0.640	1597	1.320	1.293	0.698	1740	1.085	1.302	1429	2880
3" N Deck	50	0.0598	4.05	8914	1.645	1.310	0.826	2060	1.645	1.645	0.896	2235	1.422	1.645	2202	4409
9/16" Form Deck	40	0.0150	0.75	968	0.011	0.011	0.036	71	0.011	0.011	0.037	74	0.011	0.011	412	700
9/16" Form Deck	40	0.0180	0.90	1151	0.013	0.013	0.045	90	0.013	0.013	0.045	90	0.013	0.013	570	982
9/16" Form Deck	40	0.0240	1.20	1507	0.017	0.017	0.060	119	0.017	0.017	0.060	119	0.017	0.017	952	1665
9/16" Form Deck	40	0.0300	1.50	1849	0.020	0.020	0.073	146	0.020	0.020	0.073	146	0.020	0.020	1416	2502
9/16" Form Deck	40	0.0360	1.80	2177	0.027	0.027	0.087	173	0.027	0.027	0.087	173	0.027	0.027	1960	3487
3" Form Deck	40	0.0295	1.68	1815	0.763	0.663	0.405	809	0.763	0.663	0.404	807	0.697	0.697	295	610
3" Form Deck	40	0.0358	2.04	2698	0.923	0.847	0.535	1068	0.923	0.847	0.532	1061	0.872	0.872	425	871
3" Form Deck	40	0.0474	2.70	4455	1.210	1.193	0.791	1578	1.210	1.187	0.780	1558	1.199	1.199	720	1458
3" Form Deck	40	0.0598	3.40	5574	1.510	1.510	1.005	2131	1.510	1.510	1.005	2131	1.510	1.510	1110	2232
2" Composite Deck	40	0.0295	1.53	1876	0.323	0.270	0.241	482	0.323	0.270	0.242	482	0.288	0.288	310	601
2" Composite Deck	40	0.0358	1.86	2327	0.387	0.347	0.322	643	0.387	0.347	0.321	640	0.360	0.360	444	859
2" Composite Deck	40	0.0474	2.46	3048	0.507	0.493	0.482	963	0.507	0.490	0.476	950	0.498	0.498	744	1438
2" Composite Deck	40	0.0598	3.09	3798	0.630	0.630	0.629	1314	0.630	0.630	0.629	1296	0.630	0.630	1141	2201

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All section properties, ASD flexural strengths, and web crippling strengths have been calculated in accordance with AISI S100-16/S2-20, SDI NC-2017 and SDI C-2017, as applicable.
[2]The design thickness is the uncoated base-metal thickness of the deck panel.
[3]Deck deflection under uniform loads can be determined using the moment of inertia, I_D, calculated as follows:
- For simple or multiple span, I_D is permitted to be equal to $(I_{xx} + 2 \cdot I_{ex})/3$ or I_{ex}.

[4]Web crippling strength is calculated assuming the deck is attached to the support.

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TABLE 2—COMPOSITE DECK SECTION PROPERTIES (PER FOOT OF WIDTH) [1]

Panel Type	F_y (ksi)	Design Thickness[2] (in)	Overall Depth h (in)	Total Weight (psf)	Flexural Strength (in-k/ft)	Composite Moment of Inertia for Deflection, I_d[3] (in⁴/ft)(Eq. A5-5)	Shear Strength of Composite Deck (k/ft) (Eq. 2.4.7c)
1.5" Composite Deck	40	0.0295	3.50	32.0	17.13	2.5983	2.14
			4.00	38.1	21.52	3.8568	2.59
			4.50	44.1	26.11	5.4704	3.08
			5.00	50.2	30.84	7.4797	3.60
			5.50	56.2	35.66	9.9248	4.16
			6.00	62.2	40.56	12.8455	4.76
	40	0.0358	3.50	32.4	20.23	2.8013	2.14
			4.00	38.4	25.48	4.1527	2.59
			4.50	44.5	30.98	5.8816	3.08
			5.00	50.5	36.65	8.0290	3.60
			5.50	56.5	42.45	10.6356	4.16
			6.00	62.6	48.34	13.7416	4.76
	40	0.0474	3.50	33.0	25.60	3.1358	2.14
			4.00	39.0	32.38	4.6431	2.59
			4.50	45.1	39.50	6.5669	3.08
			5.00	51.1	46.88	8.9493	3.60
			5.50	57.2	54.43	11.8321	4.16
			6.00	63.2	62.13	15.2563	4.76
	40	0.0598	3.50	33.7	30.98	3.4529	2.14
			4.00	39.7	39.31	5.1094	2.59
			4.50	45.8	48.12	7.2213	3.08
			5.00	51.8	57.26	9.8322	3.60
			5.50	57.8	66.65	12.9850	4.16
			6.00	63.9	76.22	16.7218	4.76

(Continued)

2" Composite Deck	40	0.0295	4.00	37.8	19.99	4.0250	3.01
			4.50	43.8	24.09	5.6309	3.48
			5.00	49.9	28.37	7.6215	3.98
			5.50	55.9	32.77	10.0370	4.50
			6.00	62.0	37.27	12.9175	5.04
			6.50	68.0	41.85	16.3025	5.61
	40	0.0358	4.00	38.1	23.70	4.3071	3.01
			4.50	44.2	28.60	6.0179	3.48
			5.00	50.2	33.73	8.1351	3.98
			5.50	56.2	39.02	10.6996	4.50
			6.00	62.3	44.43	13.7516	5.04
			6.50	68.3	49.94	17.3312	5.61
	40	0.0474	4.00	38.7	30.27	4.7886	3.01
			4.50	44.8	36.61	6.6772	3.48
			5.00	50.8	43.27	9.0109	3.98
			5.50	56.8	50.15	11.8310	4.50
			6.00	62.9	57.21	15.1788	5.04
			6.50	68.9	64.40	19.0949	5.61
	40	0.0598	4.00	39.3	31.21	5.2493	3.01
			4.50	45.4	37.82	7.3094	3.48
			5.00	51.4	44.78	9.8531	3.98
			5.50	57.5	52.00	12.9230	4.50
			6.00	63.5	59.41	16.5612	5.04
			6.50	69.6	66.98	20.8090	5.61

(Continued)

3" Composite Deck	40	0.0295	5.00	44.0	25.21	7.2814	3.72
			5.50	50.0	29.40	9.5433	4.20
			6.00	56.1	33.81	12.2378	4.70
			6.50	62.1	38.39	15.4059	5.22
			7.00	68.1	43.10	19.0881	5.76
			7.50	74.2	47.91	23.3242	6.33
	40	0.0358	5.00	44.3	29.96	7.7619	3.72
			5.50	50.4	34.97	10.1576	4.20
			6.00	56.4	40.24	13.0105	4.70
			6.50	62.5	45.72	16.3617	5.22
			7.00	68.5	51.37	20.2521	5.76
			7.50	74.5	57.15	24.7219	6.33
	40	0.0474	5.00	45.0	38.33	8.5771	3.72
			5.50	51.0	44.77	11.1998	4.20
			6.00	57.1	51.58	14.3225	4.70
			6.50	63.1	58.68	17.9873	5.22
			7.00	69.2	66.02	22.2356	5.76
			7.50	75.2	73.55	27.1081	6.33
	40	0.0598	5.00	45.7	46.83	9.3713	3.72
			5.50	51.7	54.73	12.2136	4.20
			6.00	57.8	63.13	15.5995	4.70
			6.50	63.8	71.91	19.5719	5.22
			7.00	69.9	81.01	24.1728	5.76
			7.50	75.9	90.35	29.4440	6.33

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All section properties, flexural strengths, and shear strengths have been calculated in accordance with SDI C-2017.
[2]The design thickness is the uncoated base-metal thickness of the deck panel.

TABLE 3A— 1.5" B DECK (40 KSI) ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3] (psf)

Span (ft)		1-Span Base Steel Thickness (in)				2-Span Base Steel Thickness (in)				3-Span Base Steel Thickness (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
5	Positive	115	143	191	242	118	149	213	269	147	185	264	333
	Negative	45	45	45	45	36	36	36	36	41	41	41	41
	L/180	106	133	183	238	255	320	442	573	199	251	346	449
5.5	Positive	95	118	158	200	98	124	177	224	122	154	219	277
	Negative	41	41	41	41	33	33	33	33	37	37	37	37
	L/180	79	100	138	179	191	240	332	430	150	188	260	337
6	Positive	80	99	133	169	83	104	149	188	103	130	185	234
	Negative	38	38	38	38	30	30	30	30	34	34	34	34
	L/180	61	77	106	138	147	185	256	331	115	145	200	260
6.5	Positive	68	85	114	144	70	89	127	161	88	111	158	200
	Negative	35	35	35	35	28	28	28	28	31	31	31	31
	L/180	48	60	83	108	116	146	201	261	91	114	157	204
7	Positive	59	73	98	124	61	77	110	139	76	96	137	173
	Negative	32	32	32	32	26	26	26	26	29	29	29	29
	L/180	39	48	67	87	93	117	161	209	73	91	126	164
7.5	Positive	51	64	86	108	53	67	96	121	66	84	119	151
	Negative	30	30	30	30	24	24	24	24	27	27	27	27
	L/180	31	39	54	70	75	95	131	170	59	74	103	133
8	Positive	45	56	75	95	47	59	84	107	58	74	105	133
	Negative	28	28	28	28	23	23	23	23	26	26	26	26
	L/180	26	32	45	58	62	78	108	140	49	61	84	110
8.5	Positive	40	50	67	84	41	52	75	95	52	65	93	118
	Negative	26	26	26	26	21	21	21	21	24	24	24	24
	L/180	22	27	37	48	52	65	90	117	41	51	70	91
9	Positive	36	44	60	75	37	47	67	85	46	58	83	105
	Negative	25	25	25	25	20	20	20	20	23	23	23	23
	L/180	18	23	31	41	44	55	76	98	34	43	59	77
9.5	Positive	32	40	53	68	33	42	60	76	41	52	75	95
	Negative	24	24	24	24	19	19	19	19	22	22	22	22
	L/180	15	19	27	35	37	47	64	83	29	37	50	65
10	Positive	29	36	48	61	30	38	54	69	37	47	68	86
	Negative	23	23	23	23	18	18	18	18	20	20	20	20
	L/180	13	17	23	30	32	40	55	72	25	31	43	56

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1] All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of L/180 and #12 HWH screws spaced 12" o.c. alternating low flutes along support points to minimum 16-gauge steel supports. Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3] Bending moment equations used for flexural stress limitations are:

Simple and Two Span: $M = wl^2/8$

Three Span or More: $M = wl^2/10$

TABLE 3B— 1.5" B DECK (50 KSI) ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3] (psf)

		1-Span				2-Span				3-Span			
		Base Steel Thickness (in)				Base Steel Thickness (in)				Base Steel Thickness (in)			
Span (ft)		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
5	Positive	137	176	236	300	143	182	245	330	177	226	303	409
	Negative	45	45	45	45	36	36	36	36	41	41	41	41
	L/180	104	130	179	233	251	313	430	561	196	245	337	440
5.5	Positive	113	146	196	248	118	151	203	274	147	187	252	340
	Negative	41	41	41	41	33	33	33	33	37	37	37	37
	L/180	78	97	134	175	188	235	323	422	148	184	253	330
6	Positive	95	123	165	209	100	127	171	231	124	158	213	287
	Negative	38	38	38	38	30	30	30	30	34	34	34	34
	L/180	60	75	103	135	145	181	249	325	114	142	195	255
6.5	Positive	81	105	141	178	85	109	146	197	106	135	182	245
	Negative	35	35	35	35	28	28	28	28	31	31	31	31
	L/180	47	59	81	106	114	142	196	256	89	111	153	200
7	Positive	70	90	121	154	74	94	126	170	92	117	157	212
	Negative	32	32	32	32	26	26	26	26	29	29	29	29
	L/180	38	47	65	85	91	114	157	205	72	89	123	160
7.5	Positive	61	79	106	134	64	82	110	149	80	102	137	185
	Negative	30	30	30	30	24	24	24	24	27	27	27	27
	L/180	31	38	53	69	74	93	128	166	58	73	100	130
8	Positive	54	69	93	118	56	72	97	131	70	90	121	163
	Negative	28	28	28	28	23	23	23	23	26	26	26	26
	L/180	25	32	44	57	61	76	105	137	48	60	82	107
8.5	Positive	48	61	82	105	50	64	86	116	62	79	107	145
	Negative	26	26	26	26	21	21	21	21	24	24	24	24
	L/180	21	26	36	47	51	64	88	114	40	50	69	90
9	Positive	42	55	74	93	45	57	77	104	56	71	96	129
	Negative	25	25	25	25	20	20	20	20	23	23	23	23
	L/180	18	22	31	40	43	54	74	96	34	42	58	75
9.5	Positive	38	49	66	84	40	51	69	93	50	64	86	116
	Negative	24	24	24	24	19	19	19	19	22	22	22	22
	L/180	15	19	26	34	37	46	63	82	29	36	49	64
10	Positive	34	44	60	76	36	46	62	84	45	58	78	105
	Negative	23	23	23	23	18	18	18	18	20	20	20	20
	L/180	13	16	22	29	31	39	54	70	25	31	42	55

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1] All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017, and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of L/180 and #12 HWH screws spaced 12" o.c. alternating low flutes along support points to minimum 16-gauge steel supports. Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3] Bending moment equations used for flexural stress limitations are:

    Simple and Two Span: $M = wl^2/8$

    Three Span or More: $M = wl^2/10$

TABLE 3C —3" N DECK (40 KSI) ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3] (psf)

Span (ft)		1-Span				2-Span				3-Span			
		Base Steel Thickness (in)				Base Steel Thickness (in)				Base Steel Thickness (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
10	Positive	55	73	103	133	62	80	113	158	77	100	140	197
	Negative	23	23	23	23	18	18	18	18	20	20	20	20
	L/180	56	70	97	127	136	169	234	307	106	132	183	241
10.5	Positive	50	66	94	121	57	73	102	144	70	91	127	179
	Negative	21	21	21	21	17	17	17	17	19	19	19	19
	L/180	49	61	84	110	117	146	202	265	92	114	158	208
11	Positive	46	60	85	110	52	66	93	131	64	83	116	163
	Negative	20	20	20	20	16	16	16	16	19	19	19	19
	L/180	42	53	73	96	102	127	176	231	80	99	138	181
11.5	Positive	42	55	78	101	47	61	85	120	59	76	106	149
	Negative	20	20	20	20	16	16	16	16	18	18	18	18
	L/180	37	46	64	84	89	111	154	202	70	87	120	158
12	Positive	39	50	72	93	43	56	78	110	54	70	98	137
	Negative	19	19	19	19	15	15	15	15	17	17	17	17
	L/180	33	41	56	74	79	98	135	178	62	77	106	139
12.5	Positive	36	47	66	85	40	52	72	102	50	64	90	127
	Negative	18	18	18	18	14	14	14	14	16	16	16	16
	L/180	29	36	50	65	70	87	120	157	54	68	94	123
13	Positive	33	43	61	79	37	48	67	94	46	59	83	117
	Negative	17	17	17	17	14	14	14	14	16	16	16	16
	L/180	26	32	44	58	62	77	106	140	48	60	83	109
13.5	Positive	30	40	57	73	34	44	62	87	43	55	77	109
	Negative	17	17	17	17	13	13	13	13	15	15	15	15
	L/180	23	29	39	52	55	69	95	125	43	54	74	98
14	Positive	28	37	53	68	32	41	58	81	40	51	72	101
	Negative	16	16	16	16	13	13	13	13	15	15	15	15
	L/180	21	26	35	46	49	62	85	112	39	48	67	88
14.5	Positive	26	35	49	64	30	38	54	76	37	48	67	94
	Negative	16	16	16	16	12	12	12	12	14	14	14	14
	L/180	18	23	32	42	45	55	77	101	35	43	60	79
15	Positive	25	32	46	59	28	36	50	71	35	45	63	88
	Negative	15	15	15	15	12	12	12	12	14	14	14	14
	L/180	17	21	29	38	40	50	69	91	32	39	54	71

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017, andSDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of L/180 and #12 HWH screws spaced 12" o.c. alternating low flutes along support points to minimum 16-gauge steel supports. Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3] Bending moment equations used for flexural stress limitations are:

Simple and Two Span: $M = wl^2/8$

Three Span or More: $M = wl^2/10$

TABLE 3D — 3" N DECK (50 KSI) ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3] (psf)

Span (ft)		1-Span Base Steel Thickness (in)				2-Span Base Steel Thickness (in)				3-Span Base Steel Thickness (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
10	Positive	66	87	127	164	72	97	138	177	89	120	172	221
	Negative	23	23	23	23	18	18	18	18	20	20	20	20
	L/180	55	69	95	124	134	166	228	299	105	130	179	234
10.5	Positive	60	79	115	149	65	88	125	161	81	109	156	201
	Negative	21	21	21	21	17	17	17	17	19	19	19	19
	L/180	48	60	82	107	115	143	197	258	90	112	155	202
11	Positive	55	72	105	136	59	80	114	147	74	100	143	183
	Negative	20	20	20	20	16	16	16	16	19	19	19	19
	L/180	42	52	71	93	100	125	172	225	79	98	134	176
11.5	Positive	50	66	96	124	54	73	105	134	68	91	130	168
	Negative	20	20	20	20	16	16	16	16	18	18	18	18
	L/180	36	45	62	82	88	109	150	197	69	86	118	154
12	Positive	46	60	88	114	50	68	96	123	62	84	120	154
	Negative	19	19	19	19	15	15	15	15	17	17	17	17
	L/180	32	40	55	72	77	96	132	173	61	75	104	136
12.5	Positive	42	55	82	105	46	62	89	114	57	78	111	142
	Negative	18	18	18	18	14	14	14	14	16	16	16	16
	L/180	28	35	49	64	68	85	117	153	54	67	92	120
13	Positive	39	51	75	97	43	58	82	105	53	72	102	131
	Negative	17	17	17	17	14	14	14	14	16	16	16	16
	L/180	25	31	43	57	61	76	104	136	48	59	81	107
13.5	Positive	36	48	70	90	40	53	76	98	49	67	95	122
	Negative	17	17	17	17	13	13	13	13	15	15	15	15
	L/180	23	28	39	50	54	67	93	122	43	53	73	95
14	Positive	34	44	65	84	37	50	71	91	46	62	88	113
	Negative	16	16	16	16	13	13	13	13	15	15	15	15
	L/180	20	25	35	45	49	61	83	109	38	47	65	85
14.5	Positive	32	41	61	78	34	46	66	85	43	58	82	106
	Negative	16	16	16	16	12	12	12	12	14	14	14	14
	L/180	18	23	31	41	44	54	75	98	34	43	59	77
15	Positive	30	39	57	73	32	43	62	79	40	54	77	99
	Negative	15	15	15	15	12	12	12	12	14	14	14	14
	L/180	16	20	28	37	40	49	68	89	31	39	53	69

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of L/180 and #12 HWH screws spaced 12" o.c. alternating low flutes along support points to minimum 16-gauge steel supports. Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3] Bending moment equations used for flexural stress limitations are:

Simple and Two Span: $M = wl^2/8$

Three Span or More: $M = wl^2/10$

TABLE 4A — 1.5" COMPOSITE DECK ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3]

Base Steel Design Thickness (in)	Overall Slab Depth h (in)	Max. Unshored Deck Span (ft-in)			Allowable Uniformly Distributed Loads (psf) Clear Span (ft-in)														
		1-Span	2-Span	3-Span	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"
0.0295	3.5	5'-11"	7'-7"	6'-10"	400	346	285	238	201	171	146	126	109	95	82	66	53	43	34
	4.0	5'-7"	7'-2"	6'-6"	400	400	361	302	255	217	186	161	139	121	105	92	81	70	59
	4.5	5'-4"	6'-10"	6'-2"	400	400	400	368	311	265	228	197	171	149	130	114	100	88	77
	5.0	5'-2"	6'-6"	5'-11"	400	400	400	400	369	315	271	234	204	178	155	136	120	105	93
	5.5	4'-12"	6'-3"	5'-8"	400	400	400	400	400	366	315	273	237	207	182	159	140	124	109
	6.0	4'-10"	5'-12"	5'-6"	400	400	400	400	400	400	360	312	272	237	208	183	161	142	126
0.0358	3.5	6'-10"	8'-6"	7'-11"	400	400	342	287	243	207	178	154	134	110	90	73	60	48	38
	4.0	6'-6"	8'-0"	7'-6"	400	400	400	364	308	264	227	197	171	150	131	116	98	81	67
	4.5	6'-3"	7'-8"	7'-2"	400	400	400	400	377	323	278	241	211	184	162	143	126	112	99
	5.0	5'-11"	7'-4"	6'-10"	400	400	400	400	400	384	331	288	251	220	194	171	151	134	119
	5.5	5'-9"	7'-0"	6'-7"	400	400	400	400	400	400	386	335	293	257	226	200	177	157	140
	6.0	5'-6"	6'-9"	6'-4"	400	400	400	400	400	400	400	384	335	295	260	230	204	181	161
0.0474	3.5	8'-4"	10'-1"	9'-7"	400	400	400	371	315	270	234	190	155	127	104	85	70	57	46
	4.0	7'-11"	9'-7"	9'-1"	400	400	400	400	400	345	298	260	227	198	164	136	113	94	78
	4.5	7'-6"	9'-1"	8'-7"	400	400	400	400	400	400	366	319	280	247	218	194	171	144	121
	5.0	7'-2"	8'-9"	8'-3"	400	400	400	400	400	400	400	381	335	295	261	232	207	185	166
	5.5	6'-11"	8'-4"	7'-11"	400	400	400	400	400	400	400	400	391	345	306	272	243	217	195
	6.0	6'-8"	8'-1"	7'-7"	400	400	400	400	400	400	400	400	400	396	351	312	279	250	224
0.0598	3.5	9'-7"	11'-4"	11'-2"	400	400	400	400	388	324	261	212	173	142	117	97	80	66	54
	4.0	9'-1"	10'-9"	10'-6"	400	400	400	400	400	400	370	323	267	221	184	153	128	107	90
	4.5	8'-8"	10'-3"	9'-11"	400	400	400	400	400	400	400	398	350	310	270	227	191	162	137
	5.0	8'-4"	9'-9"	9'-6"	400	400	400	400	400	400	400	400	400	371	330	294	264	231	197
	5.5	7'-12"	9'-5"	9'-1"	400	400	400	400	400	400	400	400	400	400	387	345	309	278	251
	6.0	7'-8"	9'-0"	8'-9"	400	400	400	400	400	400	400	400	400	400	400	397	356	320	289

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of the lesser of L/180 or ¾". Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3]Design is based on simple span analysis – continuous slab is assumed to crack over each support and to carry load as a series of simple spans.

TABLE 4B — 2" COMPOSITE DECK ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3]

Base Steel Design Thickness (in)	Overall Slab Depth h (in)	Max. Unshored Deck Span (ft-in)			Allowable Uniformly Distributed Loads (psf) Clear Span (ft-in)															
		1-Span	2-Span	3-Span	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"
0.0295	4.0	6'-10"	8'-1"	7'-10"	400	332	278	234	199	170	147	127	110	95	83	72	63	55	48
	4.5	6'-6"	7'-6"	7'-5"	400	400	336	284	242	207	178	154	134	117	102	89	78	68	59
	5.0	6'-3"	6'-11"	7'-1"	400	400	398	336	286	246	212	184	160	139	122	106	93	81	71
	5.5	5'-9"	6'-4"	6'-4"	400	400	400	390	332	285	246	214	186	163	142	125	109	96	84
	6.0	5'-2"	5'-10"	5'-9"	400	400	400	400	380	326	282	245	213	187	163	143	126	111	97
	6.5	4'-8"	5'-4"	5'-3"	400	400	400	400	400	368	318	276	241	211	185	163	143	126	111
0.0358	4.0	8'-3"	9'-4"	9'-6"	400	400	336	284	243	209	181	157	137	120	105	92	81	71	58
	4.5	7'-10"	8'-10"	8'-12"	400	400	400	345	295	254	220	191	167	147	129	113	100	88	78
	5.0	7'-6"	8'-6"	8'-7"	400	400	400	400	350	301	261	227	199	175	154	136	120	106	94
	5.5	7'-2"	8'-1"	8'-2"	400	400	400	400	400	350	304	265	232	204	180	159	140	124	110
	6.0	6'-11"	7'-10"	7'-11"	400	400	400	400	400	400	348	303	266	234	206	183	162	143	127
	6.5	6'-8"	7'-6"	7'-7"	400	400	400	400	400	400	392	343	301	265	234	207	183	163	145
0.0474	4.0	10'-5"	11'-4"	11'-8"	400	400	400	373	320	277	241	210	185	163	142	119	99	82	68
	4.5	9'-11"	10'-9"	11'-2"	400	400	400	400	389	337	293	257	226	199	177	157	140	124	105
	5.0	9'-6"	10'-4"	10'-8"	400	400	400	400	400	400	348	305	269	238	211	188	167	150	134
	5.5	9'-2"	9'-11"	10'-3"	400	400	400	400	400	400	400	356	314	277	246	219	196	175	157
	6.0	8'-10"	9'-6"	9'-10"	400	400	400	400	400	400	400	400	360	319	283	252	226	202	181
	6.5	8'-7"	9'-2"	9'-6"	400	400	400	400	400	400	400	400	400	360	320	286	256	229	206
0.0598	4.0	11'-2"	13'-2"	13'-2"	400	400	400	385	331	286	249	218	191	169	149	133	111	93	78
	4.5	10'-8"	12'-6"	12'-8"	400	400	400	400	400	349	304	266	234	207	183	163	145	130	116
	5.0	10'-3"	11'-12"	12'-3"	400	400	400	400	400	400	362	317	279	247	219	195	174	156	140
	5.5	9'-10"	11'-6"	11'-11"	400	400	400	400	400	400	400	370	327	289	257	229	205	183	164
	6.0	9'-6"	11'-1"	11'-5"	400	400	400	400	400	400	400	400	375	333	296	264	236	212	190
	6.5	9'-3"	10'-8"	11'-1"	400	400	400	400	400	400	400	400	400	377	335	299	268	241	216

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of the lesser of L/180 or ¾". Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3]Design is based on simple span analysis – continuous slab is assumed to crack over each support and to carry load as a series of simple spans.

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TABLE 4C — 3" COMPOSITE DECK ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3]

Base Steel Design Thickness (in)	Overall Slab Depth h (in)	Max. Unshored Deck Span (ft-in)			Allowable Uniformly Distributed Loads (psf) Clear Span (ft-in)														
		1-Span	2-Span	3-Span	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"	10'-6"	11'-0"	11'-6"	12'-0"	12'-6"	13'-0"	13'-6"	14'-0"
0.0295	5.0	6'-7"	7'-7"	7'-4"	299	255	219	189	164	142	124	108	95	83	73	64	55	48	42
	5.5	5'-10"	6'-8"	6'-5"	350	298	256	221	192	167	146	128	112	98	86	75	66	58	50
	6.0	5'-2"	5'-11"	5'-9"	400	345	296	256	222	194	169	148	130	114	100	88	77	68	59
	6.5	4'-8"	5'-4"	5'-2"	400	393	338	292	254	221	194	170	149	131	116	102	89	78	68
	7.0	4'-3"	4'-10"	4'-9"	400	400	381	330	287	250	219	192	169	149	131	116	102	90	78
	7.5	3'-11"	4'-6"	4'-4"	400	400	400	368	320	280	245	216	190	167	148	130	115	101	89
0.0358	5.0	10'-10"	10'-10"	11'-3"	363	311	268	232	202	177	155	137	121	107	94	84	74	65	58
	5.5	10'-4"	9'-11"	10'-4"	400	364	314	272	237	208	183	161	142	126	112	99	88	78	69
	6.0	9'-9"	9'-1"	9'-6"	400	400	363	315	275	241	212	187	165	146	130	115	102	91	80
	6.5	8'-10"	8'-5"	8'-10"	400	400	400	359	314	275	242	214	189	168	149	133	118	105	93
	7.0	8'-0"	7'-10"	8'-2"	400	400	400	400	354	311	274	242	215	190	169	151	134	119	106
	7.5	7'-5"	7'-4"	7'-8"	400	400	400	400	396	348	306	271	240	214	190	169	151	135	120
0.0474	5.0	12'-9"	13'-10"	14'-3"	400	400	354	309	271	238	211	187	166	148	132	119	106	95	85
	5.5	12'-4"	13'-2"	13'-8"	400	400	400	362	317	280	247	220	196	175	156	140	126	113	101
	6.0	11'-12"	12'-8"	13'-1"	400	400	400	400	367	324	287	255	227	203	182	163	146	132	118
	6.5	11'-8"	12'-2"	12'-7"	400	400	400	400	400	370	328	292	260	233	209	187	168	152	136
	7.0	11'-5"	11'-9"	12'-2"	400	400	400	400	400	400	371	330	295	264	236	213	191	172	155
	7.5	11'-1"	11'-4"	11'-9"	400	400	400	400	400	400	400	370	330	296	265	239	215	194	175
0.0598	5.0	13'-5"	16'-1"	15'-9"	400	400	400	386	340	300	267	238	212	190	171	154	139	121	104
	5.5	13'-0"	15'-4"	15'-3"	400	400	400	400	399	353	313	279	250	224	202	182	164	148	134
	6.0	12'-8"	14'-9"	14'-10"	400	400	400	400	400	400	363	324	290	260	234	212	191	173	157
	6.5	12'-4"	14'-2"	14'-6"	400	400	400	400	400	400	400	371	332	299	269	243	220	199	181
	7.0	12'-1"	13'-9"	14'-2"	400	400	400	400	400	400	400	400	376	338	305	276	250	226	206
	7.5	11'-10"	13'-3"	13'-8"	400	400	400	400	400	400	400	400	400	380	342	310	281	255	231

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1] All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of the lesser of L/180 or ¾". Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3] Design is based on simple span analysis – continuous slab is assumed to crack over each support and to carry load as a series of simple spans.

TABLE 5A — 9/16" FORM DECK (40 KSI) ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3] (psf)

Span (ft)		1-Span Base Steel Thickness (in)					2-Span Base Steel Thickness (in)					3-Span Base Steel Thickness (in)				
		0.0150	0.0180	0.0240	0.0300	0.0360	0.0150	0.0180	0.0240	0.0300	0.0360	0.0150	0.0180	0.0240	0.0300	0.0360
1.5	Positive	248	312	414	509	601	256	310	409	503	594	316	382	504	620	731
	Negative	114	137	150	150	150	91	110	120	120	120	104	125	136	136	136
	L/180	288	345	432	518	691	693	832	1040	1248	1664	543	652	815	977	1303
2	Positive	141	177	235	289	341	146	177	233	287	339	181	219	289	356	420
	Negative	86	103	113	113	113	68	82	90	90	90	78	93	102	102	102
	L/180	121	146	182	219	291	292	351	439	526	702	229	275	344	412	550
2.5	Positive	90	114	151	186	219	94	114	150	185	218	117	142	187	230	271
	Negative	68	82	90	90	90	55	66	72	72	72	62	75	82	82	82
	L/180	62	75	93	112	149	150	180	225	270	359	117	141	176	211	282
3	Positive	63	79	105	129	153	66	79	105	129	152	82	99	131	161	189
	Negative	57	68	75	75	75	46	55	60	60	60	52	62	68	68	68
	L/180	36	43	54	65	86	87	104	130	156	208	68	81	102	122	163
3.5	Positive	46	58	77	95	112	48	58	77	95	112	60	73	96	118	140
	Negative	48	59	64	64	64	39	47	51	51	51	44	53	58	58	58
	L/180	23	27	34	41	54	55	65	82	98	131	43	51	64	77	103
4	Positive	35	45	59	73	86	37	45	59	73	86	46	56	74	91	107
	Negative	37	45	56	56	56	34	41	45	45	45	39	47	51	51	51
	L/180	15	18	23	27	36	37	44	55	66	88	29	34	43	52	69
4.5	Positive	28	35	47	58	68	29	35	47	58	68	37	44	58	72	85
	Negative	29	35	47	50	50	28	35	40	40	40	35	42	45	45	45
	L/180	11	13	16	19	26	26	31	39	46	62	20	24	30	36	48
5	Positive	23	29	38	47	55	24	29	38	47	55	30	36	47	58	69
	Negative	24	29	38	45	45	23	29	36	36	36	28	36	41	41	41
	L/180	8	9	12	14	19	19	22	28	34	45	15	18	22	26	35
5.5	Positive	19	24	31	39	46	20	24	31	39	46	25	30	39	48	57
	Negative	20	24	31	39	41	19	24	31	33	33	23	30	37	37	37
	L/180	6	7	9	11	14	14	17	21	25	34	11	13	17	20	26
6	Positive	16	20	26	32	38	17	20	26	32	38	21	25	33	41	48
	Negative	17	20	26	32	38	16	20	26	30	30	20	25	33	34	34
	L/180	4	5	7	8	11	11	13	16	19	26	8	10	13	15	20

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.

[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of L/180 and #12 HWH screws spaced 12" o.c. alternating low flutes along support points to minimum 16-gauge steel supports. Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.

[3] Bending moment equations used for flexural stress limitations are:
    Simple and Two Span: $M = wl^2/8$
    Three Span or More: $M = wl^2/10$

TABLE 5B — 3" FORM DECK (40 KSI) ALLOWABLE UNIFORMLY DISTRIBUTED LOADS[1,2,3] (psf)

Span (ft)		1-Span Base Steel Thickness (in)				2-Span Base Steel Thickness (in)				3-Span Base Steel Thickness (in)			
		0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598	0.0295	0.0358	0.0474	0.0598
6	Positive	98	142	240	370	81	116	194	298	92	132	221	338
	Negative	38	38	38	38	30	30	30	30	34	34	34	34
	L/180	282	353	485	611	679	850	1169	1472	532	666	916	1153
6.5	Positive	91	131	221	342	75	107	179	275	85	122	204	312
	Negative	35	35	35	35	28	28	28	28	31	31	31	31
	L/180	222	278	382	481	534	669	919	1158	418	524	720	907
7	Positive	84	122	206	317	70	100	167	255	79	113	189	290
	Negative	32	32	32	32	26	26	26	26	29	29	29	29
	L/180	178	222	306	385	428	535	736	927	335	419	577	726
7.5	Positive	79	113	192	296	65	93	156	238	74	106	177	271
	Negative	30	30	30	30	24	24	24	24	27	27	27	27
	L/180	144	181	248	313	348	435	598	754	272	341	469	590
8	Positive	74	106	180	262	61	87	146	223	69	99	166	254
	Negative	28	28	28	28	23	23	23	23	26	26	26	26
	L/180	119	149	205	258	286	359	493	621	224	281	386	486
8.5	Positive	69	100	169	232	57	82	137	210	65	93	156	239
	Negative	26	26	26	26	21	21	21	21	24	24	24	24
	L/180	99	124	171	215	239	299	411	518	187	234	322	406
9	Positive	66	95	154	207	54	77	130	198	62	88	147	225
	Negative	25	25	25	25	20	20	20	20	23	23	23	23
	L/180	84	105	144	181	201	252	346	436	158	197	271	342
9.5	Positive	62	90	138	186	51	73	123	185	58	83	140	214
	Negative	24	24	24	24	19	19	19	19	22	22	22	22
	L/180	71	89	122	154	171	214	294	371	134	168	231	290
10	Positive	59	84	125	168	49	70	117	167	55	79	133	203
	Negative	23	23	23	23	18	18	18	18	20	20	20	20
	L/180	61	76	105	132	147	184	252	318	115	144	198	249
10.5	Positive	56	77	113	153	46	66	111	152	53	75	126	189
	Negative	21	21	21	21	17	17	17	17	19	19	19	19
	L/180	53	66	91	114	127	159	218	275	99	124	171	215
11	Positive	53	70	103	140	44	63	102	139	50	72	120	172
	Negative	20	20	20	20	16	16	16	16	19	19	19	19
	L/180	46	57	79	99	110	138	190	239	86	108	149	187
11.5	Positive	48	64	95	128	42	61	93	127	48	69	115	158
	Negative	20	20	20	20	16	16	16	16	18	18	18	18
	L/180	40	50	69	87	96	121	166	209	76	95	130	164
12	Positive	44	59	87	117	41	58	86	117	46	66	107	145
	Negative	19	19	19	19	15	15	15	15	17	17	17	17
	L/180	35	44	61	76	85	106	146	184	66	83	114	144
12.5	Positive	41	54	80	108	39	54	79	108	44	63	98	134
	Negative	18	18	18	18	14	14	14	14	16	16	16	16
	L/180	31	39	54	68	75	94	129	163	59	74	101	128

(Continued)

13	Positive	38	50	74	100	38	50	73	100	43	61	91	124
	Negative	17	17	17	17	14	14	14	14	16	16	16	16
	L/180	28	35	48	60	67	84	115	145	52	65	90	113
13.5	Positive	35	47	69	93	35	46	68	93	41	57	84	115
	Negative	17	17	17	17	13	13	13	13	15	15	15	15
	L/180	25	31	43	54	60	75	103	129	47	58	80	101
14	Positive	33	43	64	86	33	43	63	86	40	53	79	107
	Negative	16	16	16	16	13	13	13	13	15	15	15	15
	L/180	22	28	38	48	53	67	92	116	42	52	72	91

For **SI** Units: 1 inch = 25.4 mm; 1 foot = 304.8 mm; 1 lb. = 4.45 N

[1]All allowable uniformly distributed loads have been calculated in accordance with AISI S100-16/S2-20 and SDI NC-2017 and SDI C-2017, as applicable.
[2] Loads shown in table are uniformly distributed superimposed loads in pounds per square foot (psf) assuming a deflection limit of L/180 and #12 HWH screws spaced 12" o.c. alternating low flutes along support points to minimum 16-gauge steel supports. Span length assumes center-to-center spacing of supports. Tabulated loads shall not be increased by assuming clear span dimensions.
[3] Bending moment equations used for flexural stress limitations are:
    Simple and Two Span: $M = wl^2/8$
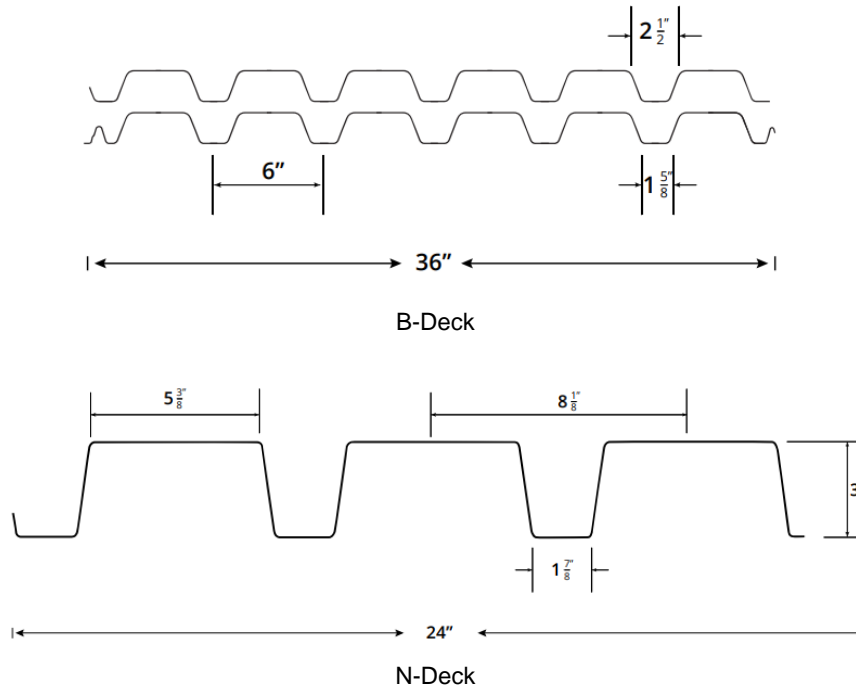   Three Span or More: $M = wl^2/10$

B-Deck

N-Deck

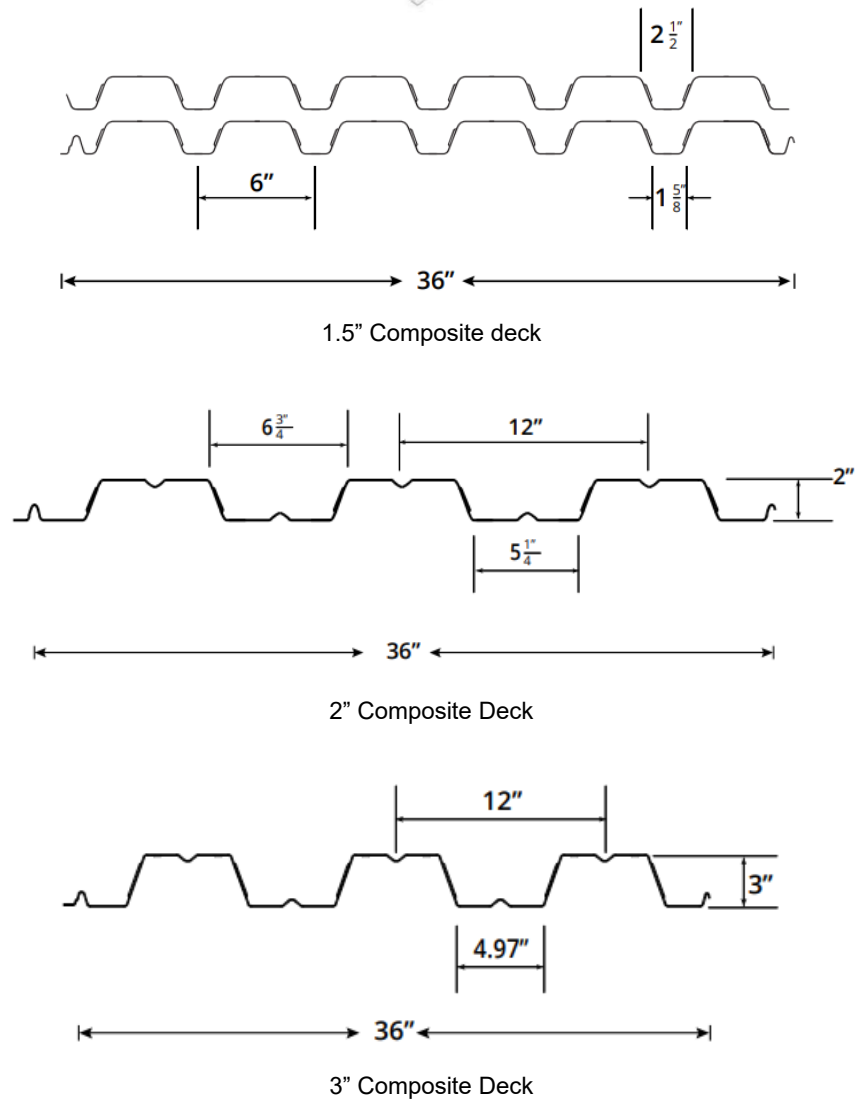**FIGURE A—B-DECK AND N-DECK PROFILES**

1.5" Composite deck

2" Composite Deck

3" Composite Deck

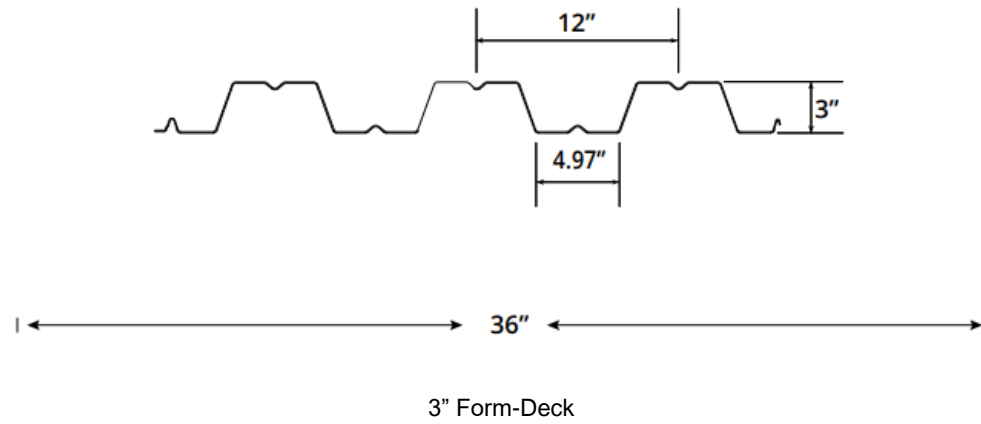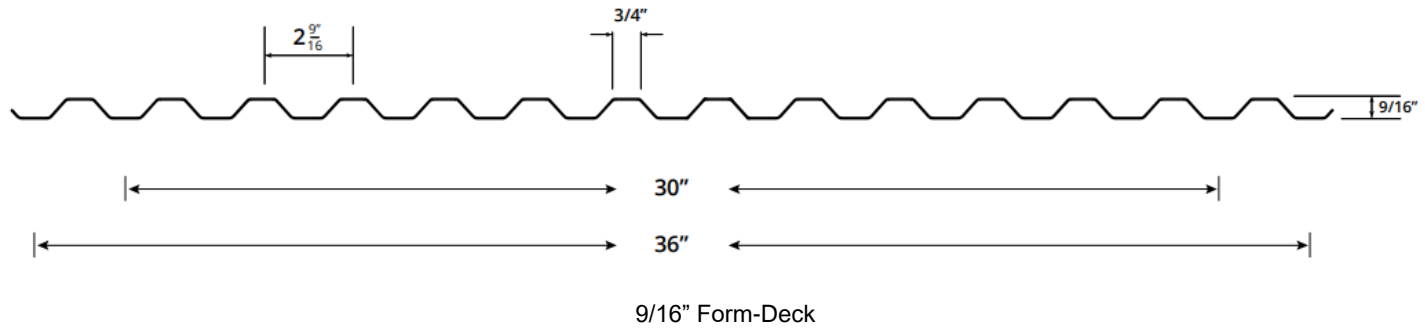**FIGURE B— COMPOSITE DECK PROFILES**

ICC-ES® *Most Widely Accepted and Trusted*

9/16" Form-Deck

3" Form-Deck

FIGURE C—FORM-DECK PROFILES